A numerical study of a particular non-conservative hyperbolic problem

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Abstract

We study the ability of several numerical schemes to solve a non-conservative hyperbolic system arising from a flow simulation of solid-liquid-gas slurries with the so-called virtual mass effect. Two classes of numerical schemes are used: some Roe-type finite volume schemes, which are based on the resolution of linearized Riemann problems, and some centered schemes with an additional artificial diffusion, such as the classical Rusanov scheme. For flow regimes of interest (steady as well as unsteady flows), several schemes do not achieve the computational process. Indeed, for such flows, the system has at least one eigenvalue having a small magnitude in the interior of the computational domain and this a possible reason for the failure of some upwind schemes using the resolution of a linearized Riemann problem. Such a failure does not appear with, for instance, the Rusanov scheme which is well known for its robustness. Furthermore, since the system is nonconservative, it is not clear what a weak solution is, when the solution is discontinuous (at least, one needs to have the nonconservative equivalent of the Rankine-Hugoniot jump conditions) and we show that the approximate solution given by different numerical schemes converges towards different “weak solutions”.

Key words: three phase flow; virtual mass effect; non-conservative hyperbolic system; FV Roe scheme with non-conservative variables

PACS:
1 Introduction

Our aim is to simulate the flow of a solid-liquid-gas slurry, which can be used, e.g., for the transport of a mixture of sand, oil and air in a pipe. The complete model is based on the so-called virtual mass effect. It can be found, for instance, in [1], [2], [3]. This model belongs to the class of non-conservative hyperbolic problems with source terms, which can be written (for one space dimension) in the form

\[
\frac{\partial w}{\partial t} + C(w) \frac{\partial w}{\partial x} = S(w), \quad x \in (0, L), \ t \in (0, T),
\]

where \( w = w(x, t) : (0, L) \times (0, T) \to \mathbb{R}^N, \ C(w) : (0, L) \times (0, T) \to \mathbb{R}^{N^2} \) is a \( N \times N \) matrix and \( S(w) : (0, L) \times (0, T) \to \mathbb{R}^N \) is a vector representing source terms. Firstly we solved this problem with the aid of the centred Rusanov scheme [4] in the papers [5] and [6]. Although systems of type (1) describe many physical processes (e.g., multiphase flow, shallow-water problems) our model is rather particular. For flow regimes of interest, various numerical schemes, using upwinding and the resolution of linearized Riemann problems, give unphysical numerical solutions and the computational processes fail. It seems to be due by the fact that at least one eigenvalue of the matrix \( C(w) \) has a small magnitude (in particular, it makes matrix \( C(W) \) very sensitive with respect to the components of \( W \) and to the rounding errors). Consequently, The first purpose of this paper is to describe this phenomenon, demonstrate it by numerical experiments and present numerical schemes which do not suffer from the mentioned effect.

The concept of a weak solution of the non-conservative system (1) is not well defined if the solution contains discontinuities. In order to define what is a solution of a non-conservative system, it is, at least, necessary to prescribe some additionally jump conditions. In the case of a conservative system, these jump conditions are implicitly given and consequently, any stable conservative scheme converges to a weak solution, which satisfies the “strong” form of the conservative system in the regular zones and the jump conditions for discontinuities.

* This work is a part of the research project MSM 0021620839 financed by MSMT and partly supported by the Grant No. 201/05/0005 of the Grant Agency of the Czech Republic.

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In our non-conservative problem, the jump conditions are not prescribed. These jump conditions could be given, for instance, with the form of a neglected viscosity term (as in [24] and [25]). But this viscosity term seems to be not available. Since the model is interesting from a practical point of view, we deal with its numerical solution although the relevant mathematical theory does not allow us to determine what a weak solution is. The numerical experiments should be used to try to give a sense what a weak solution should be. The second interest of this paper is to remark that for the non-conservative system the limit of approximate solutions given by a stable scheme may depend on this scheme. More precisely, the limit of approximate solutions (given by different schemes) differ from the speed of propagation of a discontinuity.

There exist a lot of papers dealing with a solution of (1) with the aid of various numerical schemes. Very popular are upwind schemes based on the solution of the local Riemann problem at each interface of the mesh. Since the computation of the exact solution of the local Riemann problem is difficult and expensive for complicated systems (and not well defined, in our case . . . ), schemes based on approximate Riemann solvers were introduced. For a survey see, e.g., [7], [8], [9], [10], [11]. The linearized approximate Riemann solver of Roe [12] was proposed in 1981 for the numerical solution of hyperbolic conservation laws. A weak formulation of Roe’s approximate Riemann solver, based on the choice of a path in the states space, has been introduced in [13]. This weak formulation was applied in order to build a Roe-averaged matrix for a conservative system governing a homogeneous equilibrium two-phase flow.

A generalization of this formulation to a hyperbolic non-conservative system modelling a two-component two-phase flow was carried out in [14]. However, the complexity of the considered model (defined in Section 2) gives no chance to apply this approach since we are not able to evaluate an integration along a suitable path in the states space. Another approach was used in [15] for the solution of a two-fluids model, for comparison see [16]. Also for this methods, the complexity of the considered model does not enable to perform some matrix operations analytically and their numerical realization leads to unsatisfactory results.

Therefore, we deal with an alternative to the Roe scheme, the “Finite volume Roe scheme” which was introduced in [17] to approximate solution of a two-phase flow. Indeed, we use a variant of this scheme, called VFRoe-ncv (“volumes finis” Roe with non-conservative variables) scheme, which was applied for the solution of the Euler equations in [18], for the solution of shallow-water equations with topography in [19] and for two-phase flow in [20]. VFRoe-ncv scheme is based on the solution of the linearized Riemann problem at each mesh interface in non-conservative variables. Similar approach was applied in [21] for a modelling of two-phase flow where a modification of the Roe scheme was used.
An application of VFRoe-ncv schemes causes some troubles. We observe that if some eigenvalue of $C$ in (1) has a small magnitude then the solution of the linearized Riemann problem can be unphysical which causes the failure of the computation process. However, we present numerical schemes which are able to solve the considered problem. Nevertheless, different numerical schemes converge to different weak solutions. It is caused by the non-conservativity of the system where additional (jump) condition on discontinuities have to be added in order to prescribe the speed of propagation of the discontinuities. (Furthermore, some entropy conditions are also useful to determine the admissibility of a discontinuity.)

The content of the rest of this paper is the following. In Section 2, we present the model with some of its properties and we also discuss boundary conditions for steady and unsteady flows. In Section 3, we present several schemes for the solution of solid-liquid-gas slurry flows. The capability of the schemes to compute steady as well as unsteady flow problems and the non-uniqueness of the limit of approximate solutions are discussed in Section 4. Finally, we give some concluding remarks at the end of the paper.

2 Considered problem

2.1 Governing equations

A description and derivation of the model of the flow of a solid-liquid-gas slurries in a pipe can be found in [2] and [3]. Based on the observation that the liquid and gas phases of the mixture adhere to solid particles, the so-called virtual mass effect is included in the model. For simplicity, we slightly modified the equation by neglecting partial derivatives of pressure with respect to space, for argumentation and details see [5].

In this paper, we deal with the following problem. Let $(0, L)$ be a computational domain representing a pipe and $T > 0$ be a final time of interest. We seek a vector function $w(x, t) : (0, L) \times (0, T) \to \mathbb{PAS} \subset \mathbb{R}^5$ such that

$$\tilde{A}(w) \frac{\partial w}{\partial t} + \tilde{f}(w) \frac{\partial w}{\partial x} + \tilde{C}(w) \frac{\partial w}{\partial x} = \tilde{S}(w), \quad x \in (0, L), \ t \in (0, T),$$

(2)

where

$$w = (U_\ell, U_s, C_\ell, C_s, P)^T,$$

(3)
\( \tilde{A}(w) = \begin{pmatrix} C_s \rho \ell & 0 & 0 & 0 & 0 \\ 0 & C_s \rho_s & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & \frac{C_s}{\rho_s \alpha_s^2} \\ 0 & 0 & -1 & -1 & \frac{C_s}{\rho_s \alpha_s^2} \end{pmatrix}, \quad \tilde{S}(w) = \begin{pmatrix} -p_\ell \left( g C_s \frac{\partial \rho_s}{\partial x} + I_\ell + I_{\ell s} \right) \\ -p_s \left( g C_s \frac{\partial \rho_s}{\partial x} - I_{\ell s} \right) \\ 0 \\ 0 \end{pmatrix} \),

\( \tilde{f}(w) = \begin{pmatrix} C_s \rho \ell U_\ell \\ C_s \rho_s U_s \\ (1 - K_{\ell s}) C_\ell U_\ell + K_{\ell s} C_\ell U_s \\ (1 - K_{gs}) C_g U_\ell + K_{gs} C_g U_s \end{pmatrix}, \quad \tilde{C}(w) = \begin{pmatrix} C_s \rho \ell U_\ell & 0 & 0 & 0 & 0 \\ 0 & C_s \rho_s U_s & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{pmatrix} \),

the indexes \( s, \ell \) and \( g \) denote the solid, liquid and gas phases, \( P \) is pressure of a slurry, \( \rho_k, C_k, K_k \) and \( a_k \) are the density, volume concentration, velocity and wave speed of \( k \)-phase, respectively. The volume concentrations satisfy the relation

\[ C_s + C_\ell + C_g = 1. \tag{4} \]

Further, introducing the virtual mass effect, the part of slurry, which moves with the velocity of solid phase \( U_s \) is given by

\[ C_s = C_s + K_{\ell s} C_\ell + K_{gs} C_g, \tag{5} \]

and the rest of slurry

\[ C_\ell = (1 - K_{\ell s}) C_\ell + (1 - K_{gs}) C_g \tag{6} \]

moves with the velocity of the liquid phase \( U_\ell \), where \( K_{\ell s} \) and \( K_{gs} \) represent a relative adhesion of the liquid and gas parts of slurry to the solid particles, respectively. The coefficients \( K_{\ell s} \) and \( K_{gs} \) are not constant but depend on the volume concentrations. In [2], [3], the following dependencies are proposed

\[ K_{\ell s} C_\ell = (K_1 + K_2 C_s) C_s, \tag{7} \]

\[ K_{gs} C_g = K_3 C_g, \]

with \( K_1 = K_3 = 0.5, K_2 = 0.25 \). Furthermore, the averaged densities are given by
\[ \rho_s = \frac{1}{C_s} (C_s \rho_s + K_{\ell s} C_{\ell} \rho_{\ell} + K_{gs} C_g \rho_g), \]
\[ \rho_{\ell} = \frac{1}{C_{\ell}} ((1 - K_{\ell s}) C_{\ell} \rho_{\ell} + (1 - K_{gs}) C_g \rho_g). \]

Moreover, the function \( z = z(x) \) in (3) defines the altitude of the pipe above a horizontal ground, \( g \) is the gravity and terms \( I_{\ell} \) and \( I_{\ell s} \) represent the hydraulic losses given by

\[ I_{\ell} = \frac{f_{\ell}}{2D} |U_{\ell}|, \]
\[ I_{\ell s} = \frac{3}{4} C_s C_D (U_{\ell} - U_s) |U_{\ell} - U_s|, \]

where the quantity \( f_{\ell} \) is the friction coefficient (supposed to be constant), \( D \) is the inner pipe diameter, \( C_D \) is the drag coefficient, \( d \) is the size of solid particles.

Finally, \( \text{PAS} \) denotes a set of \textit{Physically Admissible Solutions} satisfying

\[ \text{PAS} = \{ \mathbf{w} = (w_1, \ldots, w_5), \ w_3 \geq 0, \ w_4 \geq 0, \ w_3 + w_4 \leq 1, \ w_5 \geq 0 \}. \]

The system of equations (2) – (3) can be rewritten in the form corresponding with (1) by

\[ \frac{\partial \mathbf{w}}{\partial t} + \mathbf{C}(\mathbf{w}) \frac{\partial \mathbf{w}}{\partial x} = \mathbf{S}(\mathbf{w}), \quad x \in (0, L), \ t \in (0, T), \]

where \( \mathbf{C}(\mathbf{w}) \equiv \tilde{A}^{-1}(\mathbf{w}) \left[ \tilde{J}(\mathbf{w}) + \tilde{C}(\mathbf{w}) \right], \ \mathbf{S}(\mathbf{w}) \equiv \tilde{A}^{-1}(\mathbf{w}) \tilde{S}(\mathbf{w}) \) and \( \tilde{J}(\mathbf{w}) \) denotes the Jacobi matrix of \( \tilde{f}(\mathbf{w}) \) which has the form

\[ \tilde{J}(\mathbf{w}) = \begin{pmatrix}
0 & 0 & K_{3} P & \bar{K}(C_s) P & \bar{C}_{\ell} \\
0 & 0 & -K_{3} P & -\bar{K}(C_s) P & \bar{C}_s \\
(1 - K_{\ell s}) C_{\ell} & K_{\ell s} C_{\ell} & U_{\ell} & (K_1 + 2K_2 C_s) \Delta U & 0 \\
(1 - K_{gs}) C_g & K_{gs} C_g & -U_{\ell} - K_{gs} \Delta U & -U_{\ell} - K_{gs} \Delta U & 0 \\
0 & C_s & 0 & U_s & 0
\end{pmatrix}, \]

where \( \bar{K}(C_s) = -1 - K_1 - 2K_2 C_s + K_3 \) and \( \Delta U = U_s - U_{\ell} \).
The inverse of matrix $\tilde{A}$ has the form

$$
\tilde{A}^{-1}(w) = \begin{pmatrix}
\frac{1}{C_{\ell} \rho_{\ell}} & 0 & 0 & 0 \\
0 & \frac{1}{C_{s} \rho_{s}} & 0 & 0 \\
0 & 0 & \frac{A_{s} + A_{g}}{A} & -\frac{A_{s}}{A} & -\frac{A_{g}}{A} \\
0 & 0 & -\frac{A_{s}}{A} & -\frac{A_{g}}{A} & \frac{A_{s} + A_{g}}{A} \\
0 & 0 & \frac{1}{A} & \frac{1}{A} & \frac{1}{A}
\end{pmatrix}, 
$$

(13)

where

$$
A_{\ell} := \frac{C_{\ell}}{\rho_{\ell} \alpha_{\ell}^2}, \quad A_{g} := \frac{C_{g}}{\rho_{g} \alpha_{g}^2}, \quad A_{s} := \frac{C_{s}}{\rho_{s} \alpha_{s}^2}, \quad A := A_{\ell} + A_{g} + A_{s}.
$$

(14)

The system of equations (11) is supplemented by an initial condition

$$
w(x, 0) = w^0(x), \quad x \in (0, L),
$$

(15)

($w^0 : (0, L) \to \text{PAS} \text{ is a given function}$) and by suitable boundary conditions formally written

$$
B(w(x, t)) = 0, \quad x = 0, \quad x = L,
$$

(16)

which are discussed in Section 2.4.

In this paper we deal with the numerical simulation of two flow regimes:

- The simulation of a steady state flow of slurry in a pipe. This flow regime corresponds to a real situation where at the inlet of the pipe, there is a reservoir of slurry with phase concentrations $C_{\ell}^I$, $C_{s}^I$ and $C_{g}^I (= 1 - C_{s}^I - C_{\ell}^I)$ and pressure $P^I$. At the outlet of the pipe, there is the second reservoir of slurry with pressure $P^O$ ($P^O < P^I$). If the outlet reservoir is empty (free outlet), we set $P^O = 0$. Based on physical considerations this flow configuration leads to a steady state solution with constant concentrations and velocities and linearly decreasing pressure.

- The simulation of the so called water hammer. Here the steady state solution from the previous case is supposed to be the initial condition and at time $t = 0$ the outlet of the pipe is closed. This effect leads to an unsteady flow when the velocities $U_{\ell}$ and $U_{s}$ are vanishing from the closed end of the pipe. Moreover, the pressure as well as the concentrations of heavier solid and liquid particles are increasing whereas the concentration of gas goes fast to zero. These changes of concentrations, velocities and pressure propagate...
inside the pipe in opposite direction as the velocity of the main steady state flow.

2.2 Remarks to the non-conservativity of the system

**Remark 1** As we mentioned at the beginning of this paper the non-conservativity of the system (11) causes a lot of troubles, namely an impossibility to easily define a weak solution. Then a natural question arises: it is possible to formulate the considered model in a conservation form? The first obstacle represents the use of the virtual mass effect which was derived in a non-conservative form, see [2], [3].

However, even in case of a simple model the reformulation of a non-conservative problem to a conservative one is possible only if some additional information is available. For simplicity, let us consider the Burgers equation in the non-conservative form

\[
\frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} = 0, \quad x \in \mathbb{R}, \; t \geq 0.
\]  

(17)

The equation (17) is equivalent (for regular solutions) with the equation

\[
\frac{\partial u}{\partial t} + \frac{\partial}{\partial x} \left( \frac{u^2}{2} \right) = 0,
\]

(18)

and assuming that \( u \geq 0 \) also with, e.g.,

\[
\frac{\partial v}{\partial t} + \frac{\partial}{\partial x} \left( \frac{2v^{3/2}}{3} \right) = 0, \quad \text{where} \; v \equiv u^2.
\]  

(19)

Although equations (18) and (19) are equivalent for a continuous solution, they are different if the solution is discontinuous since both equations correspond to different Rankine-Hugoniot conditions. Thus the non-conservative equation (17) can be written uniquely in a conservative form only if some additional information (defining uniquely the Rankine-Hugoniot conditions) is given. But such a type of information (following from a physical aspect) is not available for the model of the three phase flow with the virtual mass effect.

**Remark 2** Let us note that if \( w \) is a discontinuous function then the non-conservative term

\[
C(w) \frac{\partial w}{\partial x}
\]

(20)
lacks meaning as the distribution and the usual theory of hyperbolic systems of conservation laws does not apply. There are several possibilities how to give a unique meaning to (20). In [22], an integration along a suitable path is used for the definition of (20). The approach presented in [23] relies on the form of matrix $C$: some equations of the system

$$\frac{\partial w}{\partial t} + C(w)\frac{\partial w}{\partial x} = 0,$$  \hspace{1cm} (21)

are considered in a strong sense and the other in a weak sense. In [24] and [25], the author adds to (21) a diffusive term $-\varepsilon \frac{\partial}{\partial x}(D(w)\frac{\partial w}{\partial x})$ ($D(w)w$ is a regular matrix) and the solution of (21) is defined as the limit of “diffusive solutions” when the diffusive term tends to zero ($\varepsilon \to 0$). However, the limit solution depends on the form of $D(w)$.

In each case some extra information is added, an integration path in [22], a type of consideration of equations in [23] or a form of a diffusive term $D$ in [24], [25]. Since problem (21) is non-conservative, the numerical treatments of the mentioned approaches lead to different numerical solutions.

Unfortunately, each of the presented approaches is too complicated to apply to the model (3) – (9). Thus, as mentioned in Section 1, we do not deal with the meaning of the non-conservative term (20) and the weak form of the system (11) any more. However, we observe that different schemes converge towards different “weak solutions” (solutions with different jump conditions).

### 2.3 Basic properties of system (11)

System (11) exhibits a strongly nonlinear non-conservative hyperbolic problem. The definition of the model (3) – (9) gives us no chance to analytically compute eigenvalues and eigenvectors of matrix $C(w)$, $w \in$ PAS. Based on numerical experiments we observe that the matrix $C(w)$, $w \in$ PAS has five real eigenvalues $\lambda_l$, $l = 1, \ldots, 5$ (if $w \notin$ PAS then $C(w)$ has complex eigenvalues in general)

$$\lambda_1 = u, \quad \lambda_{2,3} = u \pm c_1, \quad \lambda_{4,5} = u \pm c_2,$$  \hspace{1cm} (22)

where $u \in (\min(U_L, U_s), \max(U_L, U_s))$ depends on velocities, concentrations and other model parameters but it is independent of pressure. Therefore $U_l = U_s$ implies $u = U_l = U_s$. The values $c_1, c_2$, $0 \leq c_1 < c_2$ are monotonically increasing functions of pressure and depend also on velocities, concentrations and model parameters. Moreover, if the pressure is equal to zero then also $c_1 = 0$ so that $\lambda_1 = \lambda_2 = \lambda_3$ and system (11) is not strictly hyperbolic.
Fig. 1. Eigenvalues of matrix $C(w(x,t))$, $x \in (0, L = 100)$ for the simulation of the water hammer at time $t = 0$ (steady state), $t = 0.002$, $t = 0.01$ and $t = 0.05$, the whole computational domain (top) and the zooms around the pipe outlet (bottom)

We observe that the matrix $C(w)$ in (11) has at least one eigenvalue with a vanishing magnitude in the interior of the computational domain for both flow regimes of interest.

- In case of a steady state flow simulation with an (almost) empty outlet reservoir ($P^O$ is small), the velocities $U^l$ and $U^s$ are positive and pressure (almost) vanishes at the outlet of the pipe. Then there exists a point $x \in (0, L)$ where $c_1 \approx u$ and consequently $\lambda_3 \approx 0$.
- In case of a simulation of the water hammer the outlet end of the pipe is closed, velocities $U^l$ and $U^s$ are vanishing from the pipe outlet and consequently $\lambda_1$ also vanishes from the outlet of the pipe.

These properties are documented in Figure 1 where the eigenvalues of the matrices $C(w^{\text{steady}}(x))$, $x \in (0, L)$ and $C(w^{\text{wh}}(x,t))$, $x \in (0, L)$ for $t = 0.002$, $t = 0.001$ and $t = 0.05$ are presented. The state vector $w^{\text{steady}}(x)$, $x \in (0, L)$ denotes the steady state solution and $w^{\text{wh}}(x,t)$, $x \in (0, L)$, $t \geq 0$ the unsteady solution of the water hammer, obviously $w^{\text{wh}}(x,0) \equiv w^{\text{steady}}(x)$, $x \in (0, L)$.

2.4 Boundary conditions

We present the boundary conditions used for a computation of the steady state and water hammer flow regimes. We assume that slurry flows from left to right and thus the inlet and the outlet of the pipe are located at $x = 0$ and $x = L$, respectively. Some components of vector $w$ have to be given by boundary conditions and the others have to be extrapolated from the interior of the computational domain. The number of prescribed components is equal to a number of negative values

$$\tilde{\lambda}_l \equiv \lambda_l n, \quad l = 1, \ldots, 5,$$

where $\lambda_l$, $l = 1, \ldots, 5$ are eigenvalues of $C(w)$, $n$ is a 1D variant of unit outer normal, i.e., $n = -1$ and $n = 1$ for left and right end points of $(0, L)$, respectively.

Let $U^l_{\text{ex}}$ and $U^s_{\text{ex}}$ denote the liquid and solid velocities extrapolated from the interior of the interval $(0, L)$ at the inlet ($x = 0$), respectively. Similarly, let
\(U_{l}^{O-\text{ex}}, U_{s}^{O-\text{ex}}, C_{l}^{O-\text{ex}}, C_{s}^{O-\text{ex}}\) and \(P^{O-\text{ex}}\) denote the velocities, volume concentrations and pressure extrapolated from the interior of the interval \((0, L)\) at the outlet \((x = L)\), respectively. We use the following boundary conditions.

- **Pipe with open ends** (steady state solution): numerical examples show (see Figure 1, left) that the number of negative values \(\lambda_{l}, l = 1, \ldots, 5\) is equal to 3 at the inlet and 1 at the outlet. Based on the physical argumentation, we prescribe a pressure and volume concentrations at the inlet and pressure at the outlet. The other components are extrapolated from the interior of the domain. It means that we put

\[
\begin{align*}
\mathbf{w}(x = 0, t) &= (U_{l}^{I-\text{ex}}, U_{s}^{I-\text{ex}}, C_{l}^{I}, C_{s}^{I}, P^{I}), & t \in (0, T), \\
\mathbf{w}(x = L, t) &= (U_{l}^{O-\text{ex}}, U_{s}^{O-\text{ex}}, C_{l}^{O-\text{ex}}, C_{s}^{O-\text{ex}}, P^{O}), & t \in (0, T),
\end{align*}
\]

where \(C_{l}^{I}, C_{s}^{I}\) and \(P^{I}\) are liquid, solid concentration and pressure prescribed at the inlet and \(P^{O}\) is prescribed at the outlet.

- **Pipe with the closed outlet** (water hammer): numerical examples show (see Figure 1) that the number of negative values \(\lambda_{l}, l = 1, \ldots, 5\) is still equal to 3 at the inlet but 2 at the outlet \((t \geq \eta, \eta \ll 1)\). Accordingly, we use the boundary condition (24) at the inlet and we prescribe (zero) velocities and extrapolate concentrations and pressure at the outlet, i.e. we put

\[
\begin{align*}
\mathbf{w}(x = 0, t) &= (U_{l}^{I-\text{ex}}, U_{s}^{I-\text{ex}}, C_{l}^{I}, C_{s}^{I}, P^{I}), & t \in (0, T), \\
\mathbf{w}(x = L, t) &= (0, 0, C_{l}^{O-\text{ex}}, C_{s}^{O-\text{ex}}, P^{O-\text{ex}}), & t \in (0, T),
\end{align*}
\]

where \(C_{l}^{I}, C_{s}^{I}\) and \(P^{I}\) are liquid, solid concentration and pressure prescribed at the inlet.

### 3 Numerical methods

In this section we present several types of finite volume schemes for the solution of (11). We investigate the ability of numerical schemes to solve three following tasks:

C1 to keep a given steady state solution,

C2 to achieve a steady state solution,

C3 to simulate the unsteady water hammer.

Let \(T_h = \{x_{i-1/2}, x_{i+1/2}, i \in J\}\) be a partition of the computational domain \((0, L)\), where \(J = \{1, 2, \ldots, M\}\) is an index set. Moreover, let \(0 = t_0 < t_1 < t_2 < \ldots < t_r = T\) be a partition of the time interval \((0, T)\). We set \(h_i \equiv x_{i+1/2} - x_{i-1/2}, i \in J\) and \(\tau_k \equiv t_{k+1} - t_k\). The mean value of \(\mathbf{w}\) over the finite
volume \([x_{i-1/2}, x_{i+1/2}]\) at the time level \(t_k\) is given by

\[
\mathbf{w}_i^k \equiv \frac{1}{h_i} \int_{x_{i-1/2}}^{x_{i+1/2}} \mathbf{w}(x, t_k) \, dx, \quad i \in J, \ k = 0, 1, \ldots, r, \tag{28}
\]

the initial state is defined by

\[
\mathbf{w}_i^0 \equiv \frac{1}{h_i} \int_{x_{i-1/2}}^{x_{i+1/2}} \mathbf{w}^0(x) \, dx, \quad j \in J, \tag{29}
\]

where \(\mathbf{w}^0\) is given by (15). Finally, let \(\mathbf{w}_0^k, \mathbf{w}_{M+1}^k, \ k = 0, \ldots, r - 1\) denote the values on "fictitious" elements obtained with the aid of boundary conditions, see Section 2.4.

### 3.1 Centred scheme with numerical diffusion

In [5], we used a modification of the so-called Rusanov scheme [4] for the solution of (11). The Rusanov scheme was used also in [20] for the simulation of two-phase flows using the two-fluid two-pressure approach. In this paper, we present the modified Rusanov scheme for the comparison with other methods. Let us define the scheme

\[
\mathbf{w}_i^{k+1} = \mathbf{w}_i^k + \frac{T_k}{h_i} \left[ C(\mathbf{w}_i^k)(\bar{\mathbf{w}}_{i+\frac{1}{2}}^k - \bar{\mathbf{w}}_{i-\frac{1}{2}}^k) - \frac{r_i^k}{2} (\mathbf{w}_{i+1}^k - 2\mathbf{w}_i^k + \mathbf{w}_{i-1}^k) \right] - h_i S_i^k, \quad i \in J, \ k = 0, 1, \ldots, r, \tag{30}
\]

where

\[
\bar{\mathbf{w}}_{i \pm \frac{1}{2}}^k = \frac{1}{2} \left( \mathbf{w}_i^k + \mathbf{w}_{i \pm 1}^k \right), \quad i \in J, \ k = 0, \ldots, r - 1, \tag{31}
\]

\[
r_i^k = \lambda_{\max}(C(\mathbf{w}_i^k)), \quad i \in J, \ k = 0, \ldots, r - 1,
\]

\[
S_i^k = S(\mathbf{w}_i^k), \quad k = 0, \ldots, r - 1,
\]

and \(\lambda_{\max}(C(\mathbf{w}_i^k))\) denotes the maximum absolute value of eigenvalues of matrix \(C(\mathbf{w}_i^k)\). The artificial viscosity term \(\mathbf{w}_{i+1}^k - 2\mathbf{w}_i^k + \mathbf{w}_{i-1}^k\) in fact corresponds to the discretization of the Laplace operator,

\[
h_i^2 \Delta u \approx \mathbf{w}_{i+1}^k - 2\mathbf{w}_i^k + \mathbf{w}_{i-1}^k. \tag{32}
\]
The advantage of scheme (30) is that we have to calculate only the maximum eigenvalue of matrix $C$ and do not need any other decompositions of $C$. Consequently, the maximal eigenvalue can be computed numerically without any essential loss of accuracy.

3.2 Non-centred upwind schemes

Further we employ finite volume schemes based on approximate Riemann solvers. It is known that in presence of source terms standard methods can fail in approximating steady or nearly steady flows, see [9]. In order to ensure the approximation of steady flows, the so-called well-balanced schemes were presented in [26]. This approach has been extended in many papers, see, e.g., [19], [27], [28] and the references therein. Since we observed some troubles for approximating of a steady state solution, we follow the approach from [26], and add to the system of equation (11) one “partial” differential equation of a known function $\sigma(x) = x$. Then we obtain the following problem

$$\frac{\partial w}{\partial t} + C(w) \frac{\partial w}{\partial x} - S(w) \frac{\partial \sigma}{\partial x} = 0,$$

(33)

$$\frac{\partial \sigma}{\partial t} = 0, \quad x \in (0, L), \quad t \in (0, T),$$

which can be written in more compact form

$$\frac{\partial W}{\partial t} + B(W) \frac{\partial W}{\partial x} = 0, \quad x \in (0, L), \quad t \in (0, T),$$

(34)

where $W$ is the vector with 6 components

$$W = \begin{pmatrix} w \\ \sigma \end{pmatrix}$$

(35)

and the block structured of $6 \times 6$ matrix $B$ is given by

$$B(W) = \begin{pmatrix} C(w) - S(w) \\ 0 & 0 \end{pmatrix}.$$  

(36)

The regular solution of problem (34) is equivalent with the solution of (11) and, moreover, system (34) is already homogeneous. The eigenvalues of matrix $B(W)$ are the eigenvalues of $C(w)$ and, additionally, the eigenvalue zero.
Another possibility how to handle the source terms is a fractional-step method. However, numerical experiments show that by using this approach it is not possible to achieve a steady-state solution.

In the following we present several types of the so-called VFRoe-ncv (“volumes finis” Roe non-conservative variables) schemes for the numerical solution of (34). We briefly introduce an application of the VFRoe-ncv scheme for a general conservative system see e.g., [18], [19], [20] and then we describe its modifications for the non-conservative system (34). Furthermore, in Section 4 we discuss their abilities to solve problems C1 – C3.

3.2.1 VFRoe-ncv schemes for conservative system

Let us consider a general conservative hyperbolic system

$$\frac{\partial U}{\partial t} + \frac{\partial f(U)}{\partial x} = 0, \quad x \in (0, L), \ t \in (0, T),$$

where $U : (0, L) \times (0, T) \to \mathbb{R}^N$ is an unknown and $f : \mathbb{R}^N \to \mathbb{R}^N$ is a given flux. A general finite volume scheme can be written in the form

$$U_{i}^{k+1} = U_{i}^{k} - \frac{\tau}{h_i} \left( \Phi_{i+1/2}^k - \Phi_{i-1/2}^k \right), \quad i = 1, \ldots, M$$

where $\Phi_{i+1/2}^k, \ i = 0, \ldots, M$ is a numerical flux through $x_{i+1/2}$ which is usually defined in the form

$$\Phi_{i+1/2}^k \equiv \Phi(U_i^k, U_{i+1}^k), \quad i = 1, \ldots, M$$

where $\Phi : \mathbb{R}^N \times \mathbb{R}^N \to \mathbb{R}^N$ is a suitable flux function.

The basic idea of the VFRoe-ncv scheme is the following. We consider a transformation of variables $U \to Y = \Psi(U)$, where $\Psi$ is a smooth function. Although the function $\Psi$ does not need to be necessary invertible (see [29]), for simplicity we assume that that there exists the inverse mapping $\Psi^{-1}$. Then system (37) reads in the non-conservative form

$$\frac{\partial Y}{\partial t} + \bar{B}(Y) \frac{\partial Y}{\partial x} = 0, \quad \text{where} \quad \bar{B} \equiv \left( \frac{D\Psi}{DU} \right) \frac{Df}{DU} \left( \frac{D\Psi}{DU} \right)^{-1}.$$
At each interface \( x_{i+1/2}, \ i \in J \) we solve the following linearized Riemann problem

\[
\frac{\partial Y}{\partial t} + \bar{B} \frac{\partial Y}{\partial x} = 0,
\]

\[
Y(x, t = 0) = \begin{cases} 
Y_L, & \text{if } x < 0, \\
Y_R, & \text{if } x > 0,
\end{cases}
\]

where \( \bar{B} \equiv B(\bar{Y}) \) and \( \bar{Y} \) is an average state depending on \( Y_L \) and \( Y_R \), and satisfying \( \bar{Y} = Y \) if \( Y_L = Y_R = Y \). Obviously, we can put \( \bar{Y} = (Y_L + Y_R)/2 \).

If no eigenvalue of \( \bar{B} \) vanishes then the solution of (41) at the interface (i.e. for \( \frac{x}{t} = 0 \)) is given by

\[
Y^*(\bar{B}, Y_L, Y_R) \equiv Y(\frac{x}{t} \to 0) = Y_L + \sum_{\lambda_p < 0} \left( l_T^p \cdot (Y_R - Y_L) \right) r_p \quad (42)
\]

\[
= Y_R - \sum_{\lambda_p > 0} \left( l_T^p \cdot (Y_R - Y_L) \right) r_p,
\]

where \( \lambda_p, \ p = 1, \ldots, N \) are real eigenvalues of \( \bar{B} \) and \( l_p \) and \( r_p, \ p = 1, \ldots, N \) are the corresponding left and right eigenvectors, respectively, normalized so that \( l_p \cdot r_p = 1, \ p = 1, \ldots, N \), for more detail see, e.g., [10]. Then the numerical flux in (39) is defined by

\[
\Phi(U_L, U_R) \equiv f(\Psi^{-1}(Y^*(\bar{B}, Y_L, Y_R))).
\]

Finally, the VFRoe-ncv scheme can be written in the form

\[
U^{k+1}_i = U^k_i - \frac{T_k}{k_i} \left( f(\Psi^{-1}(Y^*_{i+1/2})) - f(\Psi^{-1}(Y^*_{i-1/2})) \right),
\]

\[
(44)
\]

for \( i = 1, \ldots, M \), where \( Y^*_{i+1/2} \equiv Y^*(\bar{B}, Y_i, Y_{i+1}), \ i = 0, \ldots, M \) and \( Y_i \equiv \Psi(U_i), \ i = 0, \ldots, M + 1 \).

There are some reasons why non-conservative variables \( Y = \Psi(W) \) are used. In [18], this approach was used in order to preserve numerically Riemann invariants through the contact discontinuity. In [19], the VFRoe-ncv approach was applied to shallow-water equations with topography. Two different choices of non-conservative variables are presented there. The first one avoids troubles related with an occurrence of dry areas and the second one is suitable in order to preserve all steady state (not only those with a vanishing velocity). In [20], a transformation to non-conservative variables was employed in order to obtain some properties on the approximate solution similar to properties of the exact solution, e.g., a positiveness of some quantities.
3.2.2 VFRoe-ncv schemes for the non-conservative system

Our aim is to apply the VFRoe-ncv scheme presented in Section 3.2.1 to the non-conservative system (34). Since one eigenvalue of $\bar{B}$ is identically equal to zero, we define two states at the interface ($\frac{x}{t} = 0$) from left and right (see Figure 2)

$$Y^{*,L}(\bar{B}, Y_L, Y_R) \equiv Y \left( \frac{x}{t} \to 0^- \right) = Y_L + \sum_{\lambda_p < 0} \left( l_p^T \cdot (Y_R - Y_L) \right) r_p$$

$$= Y_R - \sum_{\lambda_p \geq 0} \left( l_p^T \cdot (Y_R - Y_L) \right) r_p,$$

$$Y^{*,R}(\bar{B}, Y_L, Y_R) \equiv Y \left( \frac{x}{t} \to 0^+ \right) = Y_L + \sum_{\lambda_p \leq 0} \left( l_p^T \cdot (Y_R - Y_L) \right) r_p$$

$$= Y_R - \sum_{\lambda_p > 0} \left( l_p^T \cdot (Y_R - Y_L) \right) r_p.$$

Moreover, we define the averaged solution

$$Y^{*,LR}(\bar{B}, Y_L, Y_R) \equiv \frac{1}{2} \left( Y^{*,L}(\bar{B}, Y_L, Y_R) + Y^{*,R}(\bar{B}, Y_L, Y_R) \right).$$

We also recall a definition of the positive and negative parts of matrix $\bar{B}$ given by

$$\bar{B}^\pm \equiv R \text{Diag}(\lambda_1^\pm, \ldots, \lambda_6^\pm)L^T,$$

where $L$ and $R$ are matrices whose columns are the left and right eigenvectors of $\bar{B}$, respectively, $\text{Diag}(\lambda_1^\pm, \ldots, \lambda_6^\pm)$ is a diagonal matrix, where $\lambda_i$, $i = 1, \ldots, 6$ are eigenvalues of $\bar{B}$ and $a^+ = \max(a, 0)$, $a^- = \min(a, 0)$.

It is possible to observe that the solutions of the local Riemann problem satisfy the relations

$$\bar{B} Y^{*,L}(\bar{B}, Y_L, Y_R) = \bar{B} Y^{*,R}(\bar{B}, Y_L, Y_R) = \bar{B} Y^{*,LR}(\bar{B}, Y_L, Y_R)$$

$$= \bar{B} Y_R - \bar{B}^+(Y_R - Y_L) = \bar{B} Y_L - \bar{B}^-(Y_R - Y_L).$$

In case that $\bar{B}$ is a regular matrix the previous relations imply that

$$Y^{*,L}(\bar{B}, Y_L, Y_R) = Y^{*,R}(\bar{B}, Y_L, Y_R),$$
which is in agreement with the existence of a unique solution of the linearized Riemann problem at \( \frac{\partial}{\partial t} = 0 \) for \( B \) having no vanishing eigenvalues.

Since the components of the state vector \( W \) are non-conservative variables it is natural to use \( Y \equiv W \) in (40). It would be possible to consider different (non-conservative) variables. However, as the matrix \( B(W) \) is defined by rather complicated relations the choice of other variables than the mentioned one will probably still increase the complexity of the problem. From the point of view that the transformation of variables is the identity (\( \Psi \equiv \text{identity} \)) the following schemes can be called as the VFRoe schemes. However, since the components of \( W \) are non-conservative we still use the notation “VFRoe-ncv schemes”.

In the following, we set \( W^k_i \equiv (w^k_i, x_i)^T, \quad i = 0, \ldots, M + 1, \quad k = 0, \ldots, r - 1, \) where \( x_i \) is the centre of interval \([x_{i-1/2}, x_{i+1/2}]\), \( i = 1, \ldots, M \) and \( x_0 = 2x_{1/2} - x_1, \quad x_{M+1} = 2x_{M+1/2} - x_M \), which denotes discrete values of \( W \) on the “fictitious” elements. It means that we put \( \sigma_L = x_{i-1} \) and \( \sigma_R = x_i \) for the Riemann problem considered at the face localized at \( x_{i+1/2} \). This choice has no influence on the solution of the Riemann problem, e.g., the choice \( \sigma_L = \sigma_R = x_{i+1/2} \) produces very similar results for the “physical” components of the solution.

Provided that we formally replace \( f(U) \) by \( B(U)U \) in (44) and take into account that \( \Psi \) is the identity, we obtain the following form of the VFRoe-ncv scheme for the non-conservative system (34) (with no vanishing eigenvalues)

\[
W_{i+1}^k = W_i^k - \frac{\tau_k}{h_i} \left[ B(W_i^{k,\ast})W_i^{k,\ast} - B(W_{i-1/2}^{k,\ast})W_{i-1/2}^{k,\ast} \right], \quad (49)
\]

for \( i \in J, \quad k = 0, \ldots, r - 1, \) where vectors \( W_i^{k,\ast}, \quad i = 0, \ldots, M, \quad k = 0, 1, \ldots, r \) are the solutions of the local Riemann problems.

Another possibility is to replace the arguments of the matrix \( B(\cdot) \) in (49) by the mean volume of \( W \) on \( K_i \), then we have

\[
W_{i+1}^k = W_i^k - \frac{\tau_k}{h_i} \left[ B(W_i^k) \left( W_i^{k,\ast} - W_{i-1/2}^{k,\ast} \right) \right]. \quad (50)
\]

Further, we can modify the scheme (50) by

\[
W_{i+1}^k = W_i^k - \frac{\tau_k}{h_i} \left[ B \left( \frac{1}{2}(W_{i+1/2}^{k,\ast} + W_{i-1/2}^{k,\ast}) \right) \left( W_i^{k,\ast} - W_{i-1/2}^{k,\ast} \right) \right] \quad (51)
\]
or by

\[ W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ \frac{1}{2} \left( B(W_{i+\frac{1}{2}}^{k,*}) + B(W_{i-\frac{1}{2}}^{k,*}) \right) \left( W_{i+\frac{1}{2}}^{k,*} - W_{i-\frac{1}{2}}^{k,*} \right) \right], \quad (52) \]

All four variants of the VFRoe-ncv schemes (49) – (52) are identical for a linear system of hyperbolic equations (i.e., if \( B \) is constant) and are well-defined. The schemes were defined rather heuristically by a formal replacing terms and some numerical analysis should be carried out. Unfortunately, the complexity of the model does not allow us to derive analytically some properties of the matrix \( B \) and therefore analyse the schemes (49) – (52). On the other hand we suppose that a numerical analysis carried out for some simplified problem will produce results which will not correspond to the original model. However, our aim is not to propose and analyses the VFRoe-ncv scheme for a general non-conservative system but to show that any variant of VFRoe-ncv scheme (using a solution of the linearized Riemann problem) can not be used for a flow simulation of solid-liquid-gas slurries with the virtual mass effect. An application of the VFRoe-ncv schemes to another non-conservative problems is quite open. In [20] the VFRoe-ncv scheme of type (52) was applied for a discretization of non-conservative terms in a modelling of two-phase flows using two-fluid two-pressure approach.

Since one eigenvalue of \( B \) in (34) is identically equal to zero, we have two possible solutions of the linearized Riemann problem and therefore with the aid of (45) – (46) we define more variants of the VFRoe-ncv schemes for (34). Let \( B_{i+1/2}^{k} \equiv B((W_{i}^{k} + W_{i+1}^{k})/2), \ i = 0, \ldots, M, \ k = 0, \ldots, r-1 \) and

\[
W_{i+\frac{1}{2}}^{k,*L} = Y^{*,L}(B_{i+1/2}^{k}, W_{i}^{k}, W_{i+1}^{k}),
\]

\[
W_{i+\frac{1}{2}}^{k,*R} = Y^{*,R}(B_{i+1/2}^{k}, W_{i}^{k}, W_{i+1}^{k}),
\]

\[
W_{i+\frac{1}{2}}^{k,*LR} = Y^{*,LR}(B_{i+1/2}^{k}, W_{i}^{k}, W_{i+1}^{k}),
\]

where \( i = 0, \ldots, M, \ k = 0, \ldots, r-1 \). Based on (52) we define the first variant of the VFRoe-ncv scheme by

\[ W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ \frac{1}{2} \left( B(W_{i+\frac{1}{2}}^{k,*L}) + B(W_{i-\frac{1}{2}}^{k,*R}) \right) \right. \]

\[ \left. \left( W_{i+\frac{1}{2}}^{k,*L} - W_{i-\frac{1}{2}}^{k,*R} \right) \right], \quad (54) \]

and the second one by

\[ W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ \frac{1}{2} \left( B(W_{i+\frac{1}{2}}^{k,*LR}) + B(W_{i-\frac{1}{2}}^{k,*LR}) \right) \right. \]

\[ \left. \left( W_{i+\frac{1}{2}}^{k,*LR} - W_{i-\frac{1}{2}}^{k,*LR} \right) \right], \quad (55) \]
where \( i \in J, \ k = 0, \ldots, r - 1 \).

Based on (50) we introduce two variants of the simplified VFRoe-ncv scheme, the first one by

\[
W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ B(W_{i}^{k}) \left( W_{i+\frac{1}{2}}^{k,s,L} - W_{i-\frac{1}{2}}^{k,s,R} \right) \right],
\]

(56)

and the second by

\[
W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ B(W_{i}^{k}) \left( W_{i+\frac{1}{2}}^{k,s,L} - W_{i-\frac{1}{2}}^{k,s,R} \right) \right],
\]

(57)

where again \( i \in J, \ k = 0, \ldots, r - 1 \). It would be possible to define additional numerical schemes based on (49) and (51) of course, but computational experiments show that the numerical results obtained by these variants behave very similarly as the schemes (54) and (55), see [6]. So that we do not introduce them.

On the other hand, we define a scheme which is a direct generalization of the pure upwind scheme for a linear system of equations. Let \( B_{k}^{i} \) be the positive and negative parts of \( B_{k}^{i} = B((W_{i}^{k} + W_{i+1}^{k})/2), \ i = 0, \ldots, M, \ k = 0, \ldots, r - 1 \) given by (47). Then we define the generalized upwind scheme

\[
W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ B_{i+\frac{1}{2}}^{k} \left( W_{i+1}^{k} - W_{i}^{k} \right) + B_{i-\frac{1}{2}}^{k} \left( W_{i}^{k} - W_{i-1}^{k} \right) \right],
\]

(58)

where \( i \in J, \ k = 0, \ldots, r - 1 \). With the aid of (48), scheme (58) can be rewritten in the equivalent form as

\[
W_{i}^{k+1} = W_{i}^{k} - \frac{\tau_{k}}{h_{i}} \left[ B_{i+\frac{1}{2}}^{k} \left( W_{i}^{k} - W_{i+\frac{1}{2}}^{k,s,P} \right) - B_{i-\frac{1}{2}}^{k} \left( W_{i-\frac{1}{2}}^{k,s,P} - W_{i}^{k} \right) \right],
\]

(59)

where \( k,s,P \) represents either \( k,s,L \) or \( k,s,R \) or \( k,s,LR \). The scheme (59) is the scheme of Chakravarthy et al. presented in [30]. Although the schemes (58) and (59) are analytically identical the numerical examples in Section 4 show that their implementations produce rather different numerical solutions.

We have introduced six non-centred numerical schemes (54) – (59) for the solution of (34). If \( B \) is a regular constant matrix (i.e. system (34) is linear) then with the aid of (48) we observe that all these schemes are identical.
3.3 Some implementation remarks

3.3.1 Choice of a time step

Rusanov scheme (30) and the non-centred schemes (54) – (59) are explicit, so that in order to ensure the stability of the schemes the time step $\tau_k$ have to be chosen according a suitable stability conditions.

Let us consider a scalar problem

$$\frac{\partial w}{\partial t} + C(w) \frac{\partial w}{\partial x} = 0,$$

(60)

where $w(x,t) : (0,L) \times (0,T) \rightarrow \mathbb{R}$ is an unknown function and $C(w) : \mathbb{R} \rightarrow \mathbb{R}$ is given. It is known fact that the necessary stability condition of explicit numerical schemes solving hyperbolic differential equations is (for an equidistant partition $h = h_i, i \in J$)

$$\tau_k \leq \frac{\text{CFL} h}{\max |C(s)|},$$

(61)

where $\max |C(s)|$ is taken over

$$s \in (\min \{ w(x,t_k), x \in (0,L) \}, \max \{ w(x,t_k), x \in (0,L) \}).$$

(62)

and CFL is a positive constant less than one (for one step methods). Therefore a natural extension of the stability condition (61) to the system (34) is the following one (see [20])

$$\tau_k \leq \text{CFL} \min_{x \in (0,L)} \frac{h_i}{\lambda_{\max}(\mathbf{B}(\mathbf{W}(x,t_k)))}, \quad k = 0, \ldots, r - 1,$$

(63)

where $\lambda_{\max}(\mathbf{B}(\mathbf{W})) \equiv \max_{i=1,\ldots,6} |\lambda_i(\mathbf{B}(\mathbf{W}))|$ and $\lambda_i(\mathbf{B}(\mathbf{W}))$, $i = 1, \ldots, 6$, denote the eigenvalues of matrix $\mathbf{B}(\mathbf{W})$.

The application of the stability condition (63) to the numerical scheme is usually done by

$$\tau_k \leq \text{CFL} \min_{i=1,\ldots,M} \frac{h_i}{\lambda_{\max}(\mathbf{B}(\mathbf{W}_i^k))}, \quad k = 0, \ldots, r - 1,$$

(64)

where the minimum is taken over all discrete values $\mathbf{W}_i^k$, $i = 1, \ldots, M$. Nevertheless, our numerical experiments show that the numerical schemes (30),
(54) – (59) are not stable under condition (64) for reasonable value of CFL (e.g., $\geq 0.1$). So that we use the stability condition

$$
\tau_k \leq \text{CFL} \min \left( \min_{i=1,\ldots,M} \frac{h_i}{\lambda_{\max} \left( B(W_{i*}^k) \right)}, \frac{(h_i + h_{i+1})/2}{\lambda_{\max} \left( B(W_{i+1/2}^k) \right)} \right),
$$

$$
k = 0, \ldots, r - 1,
$$

(65)

where the minimum is taken not only over all discrete values $W_{i*}^k, i = 1, \ldots, M$ but also over the “middle states”, i.e., the solution of the linearized Riemann problems given by (53). The condition (65) is not sufficient to guarantee (63) but it takes into account more information than (64). Numerical experiments show that using (63) the presented numerical schemes have better stability properties, namely some of them are stable under CFL $\leq 0.9$ for steady-state problems and under CFL $\leq 0.4$ for unsteady problems. In Section 4.2.5 we compare the size of time steps given by the conditions (64) and (65).

### 3.3.2 Linear algebra operations

We have mentioned in Section 2.3 that the eigenvalues and eigenvectors of the matrix $B$ have to be computed numerically. We use the software package LAPACK [31] in the double precision mode, which is optimized with respect to rounding errors and CPU-time. With the aid of LAPACK’s subroutines we compute eigenvalues and left and right eigenvectors of $B$ and then the solution of linearized Riemann problems $Y^*_{\text{L}}(B, \cdot, \cdot), Y^*_{\text{R}}(B, \cdot, \cdot)$. The matrix decompositions $B^\pm$ are computed just according relations (45) and (47), respectively.

### 3.4 Schemes of higher order

#### 3.4.1 Higher order in space

The proposed numerical schemes have formally first-order accurate on meshes with a constant mesh size. A natural way to increase the order of accuracy is to apply a higher order reconstruction technique, which is widely used in finite volumes methods, see e.g., [32]. We describe here a higher order extension only of scheme (58), for the other schemes it can be done analogously.

With the aid of scheme (58) we define a higher order scheme based on the MUSCL approach with a slope limiter by

21
\[ W_{i+1}^k = W_i^k - \frac{\tau_k}{h_i} \left[ B_{i+\frac{1}{2}}^k \left( W_{i+\frac{1}{2},-}^k - \hat{W}_{i-\frac{1}{2},-}^k \right) \right. \]
\[ \left. + B_{i-\frac{1}{2}}^k \left( \hat{W}_{i+\frac{1}{2},+}^k - \hat{W}_{i-\frac{1}{2},+}^k \right) \right], \]
\[ i \in 1, \ldots, M, \; k = 0, \ldots, r - 1. \]

where

\[ \hat{W}_{i+\frac{1}{2},-}^k = W_i^k + \frac{h_i}{2} P_i^k, \quad i = 1, \ldots, M, \; k = 0, \ldots, r - 1, \] (67)
\[ \hat{W}_{i+\frac{1}{2},+}^k = W_{i+1}^k - \frac{h_{i+1}}{2} P_{i+1}^k \quad i = 0, \ldots, M - 1, \; k = 0, \ldots, r - 1, \]

and \( P_i^k \in \mathbb{R}^6, \; i = 1, \ldots, M, \; k = 0, \ldots, r - 1 \) are suitable slopes. The state vectors \( \hat{W}_{\frac{1}{2},-}^k, \; \hat{W}_{M+\frac{1}{2},+}^k, \; k = 0, \ldots, r - 1 \) are obtained from the boundary conditions. If \( P_i^k = 0, \; i = 1, \ldots, M, \; k = 0, \ldots, r - 1 \), then scheme (66) reduces to (58).

For the slopes we use the following reconstruction with a limiter

\[ P_i^k = \minmod \left( \frac{W_{i+1}^k - W_i^k}{2h}, c \frac{W_{i+1}^k - W_i^k}{h}, c \frac{W_i^k - W_{i-1}^k}{h} \right), \] (68)
\[ i = 2, \ldots, M - 1, \]
\[ P_1^k = c \frac{W_2^k - W_1^k}{h}, \]
\[ P_M^k = c \frac{W_M^k - W_{M-1}^k}{h}, \]

where \( c \) is a suitable constant and

\[ \minmod(a_1, a_2, a_3) = \begin{cases} 
\text{s min } i=1,2,3 \; |a_i| & \text{if } s = \text{sign}(a_i), \; i = 1, 2, 3, \\
0 & \text{otherwise,} \end{cases} \] (69)

where \( \text{sign}(a) \) denotes the signum function. The condition \( s = \text{sign}(a_i), \; i = 1, 2, 3 \) means that all three numbers have to have the same signum.

The choice \( c = 2 \) in (68) ensure that the numerical scheme is total variation diminishing (TVD) by mean values and the choice \( c = 1 \) ensures that the numerical scheme is TVD. If a scheme of the second order with respect to time is used it is often better to take \( c = 1 \) instead of \( c = 2 \). In some cases, namely for unsteady flows, it is suitable to choose \( c < 1 \) for the definitions of \( P_1^k \) and \( P_M^k \) which avoids some unphysical behaviour of solutions.
Another possibility is an application the approach of high-resolution central schemes from [33] to the Rusanov scheme.

3.4.2 Higher order discretization in time

The numerical scheme (66) – (69) is only first-order accurate in time. Therefore we applied the following second order Runge-Kutta method. Let the first order scheme be written formally in the form

\[ W_{i}^{k+1} = W_{i}^{k} - \tau_{k} F(W_{i-1}^{k}, W_{i}^{k}, W_{i+1}^{k}), \quad i = 1, \ldots, M, \]  

(70)

for \( k = 0, \ldots, r - 1 \), where \( F(W_{i-1}^{k}, W_{i}^{k}, W_{i+1}^{k}) \) denotes numerical fluxes depending on the values of \( W \) within element \( i \) and its neighbours at the time level \( t_{k} \). Then the second order Runge-Kutta scheme can be written in the form

\[
W_{i}^{k+1/2} = W_{i}^{k} - \frac{\tau_{k}}{2} F(W_{i-1}^{k}, W_{i}^{k}, W_{i+1}^{k}), \quad i = 1, \ldots, M,
\]

(71)

\[
W_{i}^{k+1} = W_{i}^{k} - \tau_{k} F(W_{i-1}^{k+1/2}, W_{i}^{k+1/2}, W_{i+1}^{k+1/2}),
\]

for \( k = 0, \ldots, r - 1 \). The application of the Runge-Kutta time discretization gives not only higher order of accuracy but it also increases the stability of the schemes, see Section 4.3.

4 Numerical examples

4.1 Problem data

In this section we discuss the ability of schemes (30), (54) – (59) to solve the tasks C1 – C3 introduced in Section 3. Using data from [3] the calculations were carried out with the following values of the model parameters in (3) – (9) corresponding to a sand (solid), water (liquid) and air (gas):

\[
\rho_{s} = 2600 \text{ kg.m}^{-3}, \quad \rho_{l} = 1000 \text{ kg.m}^{-3}, \quad \rho_{g} = 1.28 \text{ kg.m}^{-3},
\]

\[
a_{s} = 7000 \text{ m.s}^{-1}, \quad a_{l} = 500 \text{ m.s}^{-1}, \quad a_{g} = 300 \text{ m.s}^{-1},
\]

(72)

\[ f_{l} = 0.04, \quad D = 0.1m, \quad C_{D} = 0.1, \quad d = 5.10^{-4}m, \quad \frac{dz}{dx} = 0 \] and the length of the pipe is \( L = 100 \text{ m} \). Moreover, the following initial and boundary conditions were used for the tasks C1 – C3 defined at the beginning of Section 3:

23
We use the boundary conditions (24) – (25) with \( C^I = 0.89 \), \( C^S = 0.1 \), \( P^I = 5.5 \cdot 10^5 \text{ Pa} \) and \( P^O = 0 \text{ Pa} \). The initial condition (15) is defined by the steady state solution

\[
\mathbf{w}_0 = \mathbf{w}^{\text{steady}} \equiv (5.049, 5.219, 0.89, 0.1, 5.5 \cdot 10^5 (1 - x/L)),
\]

which corresponds numerically but can be easily verified analytically. We investigate the ability of the schemes to keep the steady state solution (73) for \( t \rightarrow \infty \).

We use the same boundary conditions as in C1 and the initial condition

\[
\mathbf{w}_0 = (6.5, 6.5, 0.89, 0.1, 5.5 \cdot 10^5 (1 - x/L)).
\]

We investigate the ability of the schemes to converge to the steady state solution (73) for \( t \rightarrow \infty \).

We investigate the ability of the schemes to simulate the unsteady water hammer. We use the boundary conditions (26) – (27) with \( C^I = 0.89 \), \( C^S = 0.1 \), \( P^I = 5.5 \cdot 10^5 \text{ Pa} \) and the (steady state) initial condition (73). We compute the solution for time interval \((0, 0.3)\) during whose the water hammer propagates up to the pipe inlet.

The comparison of the numerical methods were performed mostly for 25 sub-intervals of \((0, L)\) \((M = 25)\), some calculations on finer meshes are presented.

### 4.2 Discussion of numerical results

Based on many numerical experiments, the following interesting but unpleasant property of the linearized Riemann solutions (45) – (46) was observed. By \( \mathbf{W} = (\mathbf{w}, \sigma)^T \in \text{PAS} \) we mean that \( \mathbf{w} \in \text{PAS} \), see (10). There exist vectors \( \mathbf{W}_L \in \text{PAS} \) and \( \mathbf{W}_R \in \text{PAS} \) such that

\[
\mathbf{Y}^*, P(\mathbf{B}((\mathbf{W}_L + \mathbf{W}_R)/2), \mathbf{W}_L, \mathbf{W}_R) \not\in \text{PAS},
\]

where \( \mathbf{Y}^*, P \) represents either \( \mathbf{Y}^*, L \) or \( \mathbf{Y}^*, R \) or \( \mathbf{Y}^*, LR \). This property was observed in regions of the computational domain \((0, L)\), where one (not identically vanishing) eigenvalue of \( \mathbf{B}((\mathbf{W}_L + \mathbf{W}_R)/2) \) is close to zero. The matrix \( \mathbf{B} \) contains one eigenvalue identically equal to zero (vanishing eigenvalue) but it does not cause any troubles in computations.

The property (75) is documented in Table 1, where examples of the solutions \( \mathbf{Y}^*, L \) and \( \mathbf{Y}^*, R \) of the linearized Riemann problem (41) for four pairs of vectors \( \mathbf{W}_L \) and \( \mathbf{W}_R \) are shown. The nonphysical quantities are bolted and table also contains the corresponding eigenvalues \( \lambda_i, i = 1, \ldots, 6 \) of the matrix \( \mathbf{B}((\mathbf{W}_L + \mathbf{W}_R)/2) \). The physical no-admissibility of \( \mathbf{Y}^*, L \) and \( \mathbf{Y}^*, R \) is probably
the main reason of the disability of the schemes using the solution of the linearized Riemann problem to solved satisfactory the tasks C1 – C3. The fact that the solution of the linearized Riemann problem can be unphysical may depend of the choice of the variable for the linearization process in VFRoe-ncv, see [34]. But for the considered model, as we mentioned in Section 3.2.2, there is no other natural choice of the variables for the linearization process than the used one. The nonphysical solutions of the linearized Riemann problem appear also for conservative systems, see, e.g, [35] where the vacuum scenario of the Euler equations is investigated. We suppose that the situation from [35] differs from those ones presented in Table 1 since the velocity directions are completely different.

In the following, we discuss the obtained numerical results for each group of the schemes separately.

4.2.1 Modified Rusanov scheme

The modified Rusanov scheme (30) gives satisfactory numerical results for all three tasks. It keeps the steady state solution in the task C1 and the solution of the task C2 do not suffer from any unphysical oscillations. Finally, Figure 3 shows the simulation of the water hammer. We observe that the Rusanov scheme yields to a slower propagation of changes of the state vector for the water hammer in comparison with other schemes, compare Figures 3 and 7.

4.2.2 VFRoe-ncv schemes (54) and (55)

Scheme (55) is not able to solve the task C1 what is documented in Figure 4, where the dependence of the velocities $U_l$ and $U_s$ at the cell with $x_i = 91$ on the number of time steps is presented. We observe that after approximately 50 time steps some instabilities of the numerical solution arise and consequently the solution blows up very fastly. This instability is not caused by a violation of a stability condition with respect to the length of the time step (the presented results were obtained with CFL = 0.005 in (63)) but we suppose that these instabilities come from a very high sensitivity of the solution of the Riemann problem with respect to rounding errors. It make no sense to apply the schemes (55) for the tasks C2 and C3.
<table>
<thead>
<tr>
<th>example</th>
<th>$W_L$</th>
<th>$W_R$</th>
<th>$Y^{*,L}$</th>
<th>$Y^{*,R}$</th>
<th>$\lambda_1, \ldots, \lambda_6$</th>
</tr>
</thead>
<tbody>
<tr>
<td>E1</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>$U_L$</td>
<td>6.500</td>
<td>6.500</td>
<td>4.457</td>
<td>6.401</td>
<td>104.7</td>
</tr>
<tr>
<td>$U_s$</td>
<td>6.500</td>
<td>6.500</td>
<td>17.223</td>
<td>6.820</td>
<td>-98.2</td>
</tr>
<tr>
<td>$C_L$</td>
<td>0.890</td>
<td>0.890</td>
<td>$1.052$</td>
<td>0.885</td>
<td>13.4</td>
</tr>
<tr>
<td>$C_s$</td>
<td>0.100</td>
<td>0.100</td>
<td>-0.055</td>
<td>0.1050</td>
<td>6.5</td>
</tr>
<tr>
<td>$P$</td>
<td>93500.</td>
<td>82500.</td>
<td>85624.</td>
<td>79369</td>
<td>-0.43</td>
</tr>
<tr>
<td>$\sigma$</td>
<td>83.</td>
<td>85.</td>
<td>83.</td>
<td>85.</td>
<td>0.</td>
</tr>
<tr>
<td>E2</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>$U_s$</td>
<td>5.670</td>
<td>4.930</td>
<td>80.672</td>
<td>19.093</td>
<td>-98.2</td>
</tr>
<tr>
<td>$C_L$</td>
<td>0.890</td>
<td>0.890</td>
<td>$1.906$</td>
<td>0.705</td>
<td>13.2</td>
</tr>
<tr>
<td>$C_s$</td>
<td>0.100</td>
<td>0.100</td>
<td>-0.882</td>
<td>0.280</td>
<td>6.0</td>
</tr>
<tr>
<td>$P$</td>
<td>115917.</td>
<td>105766</td>
<td>67914.</td>
<td>126308</td>
<td>-2.3</td>
</tr>
<tr>
<td>$\sigma$</td>
<td>79.</td>
<td>81.</td>
<td>79.</td>
<td>81.</td>
<td>0.</td>
</tr>
<tr>
<td>E3</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>$U_L$</td>
<td>6.963</td>
<td>7.259</td>
<td>-147.844</td>
<td>-11.931</td>
<td>107.1</td>
</tr>
<tr>
<td>$U_s$</td>
<td>3.177</td>
<td>0.221</td>
<td>477.934</td>
<td>302.555</td>
<td>-98.3</td>
</tr>
<tr>
<td>$C_L$</td>
<td>0.890</td>
<td>0.890</td>
<td>$7.638$</td>
<td>-2.744</td>
<td>9.8</td>
</tr>
<tr>
<td>$C_s$</td>
<td>0.100</td>
<td>0.100</td>
<td>-6.466</td>
<td>3.872</td>
<td>-5.5</td>
</tr>
<tr>
<td>$P$</td>
<td>117170.</td>
<td>105652</td>
<td>655575.</td>
<td>791811</td>
<td>4.0</td>
</tr>
<tr>
<td>$\sigma$</td>
<td>79.</td>
<td>81.</td>
<td>79.</td>
<td>81.</td>
<td>0.</td>
</tr>
<tr>
<td>E4</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>$U_L$</td>
<td>3.563</td>
<td>0.</td>
<td>6.651</td>
<td>27.828</td>
<td>14.991</td>
</tr>
<tr>
<td>$U_s$</td>
<td>4.317</td>
<td>0.</td>
<td>-13.841</td>
<td>19.019</td>
<td>-6.480</td>
</tr>
<tr>
<td>$C_L$</td>
<td>0.899</td>
<td>0.899</td>
<td>0.7282</td>
<td>250.115</td>
<td>3.9</td>
</tr>
<tr>
<td>$C_s$</td>
<td>0.102</td>
<td>0.102</td>
<td>0.272</td>
<td>1.692</td>
<td>0.2</td>
</tr>
<tr>
<td>$P$</td>
<td>214037.</td>
<td>214037</td>
<td>250072.</td>
<td>-6.15E+10</td>
<td>0.2</td>
</tr>
<tr>
<td>$\sigma$</td>
<td>98.</td>
<td>102.</td>
<td>98.</td>
<td>102.</td>
<td>0.</td>
</tr>
</tbody>
</table>

Table 1

Examples of solutions $Y^{*,L}$ and $Y^{*,R}$ of the linearized Riemann problem for four pairs of $Y_L$ and $Y_R$ and the corresponding eigenvalues $\lambda_i$, $i = 1, \ldots, 6$, physically no admissible quantities are bolted.

Fig. 4. Task C1, $M = 50$, VF Roe-ncv scheme (55), dependence of $U_L$ and $U_s$ at the cell with $x_i = 91$ on the number of time steps
Scheme (54) is able to solve the task C1 but not the task C2. The failures of the scheme is caused by the property (75) since \( B(W_{i+\frac{1}{2}}^{k,*}) \) becomes meaningless for \( W_{i+\frac{1}{2}}^{k,*} \notin \text{PAS} \). Here \(*,L, R\) represent \(*,L\) or \(*,R\) or \(*,LR\) and \(i\) corresponds to the finite volume with the unphysical state vector. The example E1 in Table 1 corresponds to the application of the scheme (55) to the task C2. It make no sense to apply the scheme (55) to the task C3.

### 4.2.3 Simplified VFRoe-ncv schemes

Schemes (56) – (57) give significantly better results than schemes (54) – (55). It is caused by the fact that solutions of linearized Riemann problems are not used as arguments of matrix \( B \) but they appear in (56) and (57) only as differences \( W_{i+\frac{1}{2}}^{k,*L} - W_{i-\frac{1}{2}}^{k,*R} \) and \( W_{i+\frac{1}{2}}^{k,*LR} - W_{i-\frac{1}{2}}^{k,*LR} \), respectively. We observed the following situation

\[
W_{i+\frac{1}{2}}^{k,*L} \notin \text{PAS} \quad \text{and} \quad W_{i-\frac{1}{2}}^{k,*R} \notin \text{PAS}, \quad \text{but} \quad (W_{i+\frac{1}{2}}^{k,*L} - W_{i-\frac{1}{2}}^{k,*R}) \in \text{PAS}, \tag{76}
\]

which causes trouble for scheme (54) but not for (56). This explains the better behaviour of (56) – (57) compared to (54) – (55).

The schemes (57) solves well the tasks C1 and C2. The scheme (56) solves well the task C1, but for the task C2 it converges to an unphysical “steady state” solution containing spurious oscillations, which are localized near a node where a no-vanishing eigenvalue is close to zero, see Figure 5. It is interesting that these oscillations are completely independent of the size of CFL number used in the stability condition and the oscillating solution represents a steady state solution form the numerical point of view, i.e., there exists \( k_0 > 0 \) such that \( w_i^{k+1} = w_i^k, \ i = 1, \ldots, M \) for any \( k \geq k_0 \).

On the other hand, both scheme (56) – (57) are able to solve the task C3, but the solutions suffer from instabilities near the pipe outlet, where one no-vanishing eigenvalue is small. These instabilities are higher for the scheme (56) and can not be suppressed by reducing the time step. Figure 6 shows the simulation of the water hammer obtained by the scheme (56), where the instabilities are observed.
4.2.4 Generalized upwind schemes

Although schemes (58) and (59) are analytically identical they give numerically identical results only for tasks C1 and C2. They solve very well C1 but for C2 converge to an unphysical steady state solution, similarly as scheme (56), see Figure 5.

However, schemes (58) – (59) are also able to solve the unsteady problem C3. Using (58) we obtain a very good simulation of the water hammer for the similar CFL number (CFL = 0.3) as the modified Rusanov scheme (30), see Figure 7. On the other hand with the aid of scheme (59) the calculation can be carried out successfully only if $\text{CFL} \leq 0.2$. This is a very important observation that two analytically identical schemes have different computational properties. This is caused probably by the fact that the scheme (59) uses the solution of linearized Riemann problems whose computation is more sensitive with respect to the rounding errors. So that smaller time step should be used in order to avoid instabilities caused probably by rounding errors. The example E4 from Table 1 corresponds to the solution of the water hammer obtained by the scheme (59) with $\text{CFL} = 0.29$ in third time step. We observe that the solution of the linearized Riemann problem at $x = 100$ (the pipe outlet) is very unphysical $P \ll 0$. On the other hand using the numerical solution obtained by the scheme (59) with $\text{CFL} = 0.29$ after two time steps as an initial condition for the scheme (58) we obtain results practically identical with Figure 7. Based (not only) on this observation we suppose that numerical schemes based on the solution of the linearized Riemann problem are not suitable for a flow simulation of solid-liquid-gas slurries.

Moreover, Figures 8 and 9 show a simulation of the water hammer carried out by scheme (58) for 200 and 800 elements, respectively. Comparing results obtained using (58) for 50, 200 and 800 elements (Figures 7, 8 and 9) we observe an interesting convergence of the solution with respect to mesh step $h$, namely for the solid volume concentrations $C_s$.

4.2.5 Comparison of the stability conditions (64) and (65)

All computations presented in Section 4 were carried out using the stability condition (65). There is a natural question whether the application of the
Fig. 7. Task C3, $M = 25$, generalized upwind scheme (58), simulation of the water hammer, distributions of $C_l$, $C_s$, $C_g$, $U_l$, $U_s$, and $P$ at $t = 0$, $t = 0.1$, $t = 0.2$ and $t = 0.3$

Fig. 8. Task C3, $M = 200$, generalized upwind scheme (58), simulation of the water hammer, distributions of $C_l$, $C_s$, $C_g$, $U_l$, $U_s$, and $P$ at $t = 0$, $t = 0.1$, $t = 0.2$ and $t = 0.3$

Fig. 9. Task C3, $M = 800$, generalized upwind scheme (58), simulation of the water hammer, distributions of $C_l$, $C_s$, $C_g$, $U_l$, $U_s$, and $P$ at $t = 0$, $t = 0.1$, $t = 0.2$ and $t = 0.3$

Fig. 10. Comparison of the size of time steps given by the stability conditions (64) (full line) and (65) (dashed line), steady state problem (left) and the water hammer (right)

simpler condition (64) gives similar results. Figure 10 shows the comparison of the size of time steps given by the stability conditions (64) and (65) for the steady-state problem and the water hammer. We observe that the “stronger” condition (65) has no influence in comparison with (64) for the steady solution but essential influence for the unsteady problem. These results were obtained with the aid of the generalized Rusanov scheme (30) but the results are very similar also for other schemes (which are able to solve C3).

4.3 Higher order schemes

Finally, we present results concerning the stability of the higher order schemes for the simulation of the water hammer. Table 2 shows the upper limit of the CFL numbers used in the stability condition (65) which ensures the stability in space (66) – (69) and time (71) for higher order variants of schemes (30) and (58). We observe that the use of the second order time discretization leads to a higher stability of the schemes. Moreover, the application of the second order in space (with the first order in time) leads to a decrease of the stability
Table 2
Stability of the higher order scheme, upper CFL numbers for the Rusanov scheme.

<table>
<thead>
<tr>
<th>Scheme</th>
<th>Order in space</th>
<th>Order in time</th>
<th>Maximal CFL</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rusanov (30)</td>
<td>1</td>
<td>1</td>
<td>0.4</td>
</tr>
<tr>
<td>Rusanov (30)</td>
<td>2</td>
<td>1</td>
<td>0.25</td>
</tr>
<tr>
<td>Rusanov (30)</td>
<td>1</td>
<td>2</td>
<td>0.55</td>
</tr>
<tr>
<td>Rusanov (30)</td>
<td>2</td>
<td>2</td>
<td>0.55</td>
</tr>
<tr>
<td>upwind1 (58)</td>
<td>1</td>
<td>1</td>
<td>0.3</td>
</tr>
<tr>
<td>upwind1 (58)</td>
<td>2</td>
<td>1</td>
<td>0.3</td>
</tr>
<tr>
<td>upwind1 (58)</td>
<td>1</td>
<td>2</td>
<td>0.75</td>
</tr>
<tr>
<td>upwind1 (58)</td>
<td>2</td>
<td>2</td>
<td>0.9</td>
</tr>
</tbody>
</table>

4.4 Summary of the results

The properties of the numerical schemes (30), (54) – (59) observed in Section 4.2 are summarised in Table 3. The numerical examples presented in Section 4.2 indicate that none of the numerical schemes (54) – (59) is able to solve the tasks C1 – C3. The best results were obtained by scheme (58) whose simulation of the water hammer looks realistic and CFL number is not too restrictive. The only drawback of (58) is that it converges to an unphysical steady state solution for the task C2. This lack can be overcome by adding a small dissipative term to the scheme, e.g.,

\[
W_{i+1}^k = W_i^k - \frac{\tau_k}{h_i} \left[ B_{i+1}^{k-1} \left( W_{i+1}^k - W_i^k \right) + B_{i-1}^{k+1} \left( W_i^k - W_{i-1}^k \right) - \frac{\epsilon_i}{2} \left( W_{i+1}^k - 2W_i^k + W_{i-1}^k \right) \right], \quad i \in J, \ k = 0, \ldots, r - 1, \tag{77}
\]

where \( \epsilon > 0 \). Numerical examples show that the value of \( \epsilon \geq 0.01 \) ensure that numerical solution of the task C2 converges to a physical steady state (73). On the other hand, the presence of artificial viscosity causes a retarding and a small smearing of the solution for the task C3. Figure 11 compares the simulation of the water hammer obtained by the Rusanov scheme (30) and scheme (77) with \( \epsilon = 1, \epsilon = 0.1 \) and \( \epsilon = 0.01 \) (= scheme (58)) with \( M = 50, M = 200 \) and \( M = 800 \), respectively (for \( \epsilon \leq 0.01 \) the results are almost identical with the results obtained with \( \epsilon = 0 \)). We observe an increase of the retardation for propagation of discontinuities for increasing \( \epsilon \).
Table 3
Properties of numerical schemes (30), (54) – (59)

<table>
<thead>
<tr>
<th>Computational task</th>
<th>C1</th>
<th>C2</th>
<th>C3</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rusanov (30)</td>
<td>OK</td>
<td>OK</td>
<td>OK</td>
</tr>
<tr>
<td>VFRoe-ncv1 (54)</td>
<td>OK</td>
<td>fails</td>
<td>–</td>
</tr>
<tr>
<td>VFRoe-ncv2 (55)</td>
<td>fails</td>
<td>fails</td>
<td>–</td>
</tr>
<tr>
<td>VFRoe-ncv-sim1 (56)</td>
<td>OK</td>
<td>OK</td>
<td>OK unstable oscillations</td>
</tr>
<tr>
<td>VFRoe-ncv-sim2 (57)</td>
<td>OK</td>
<td>OK</td>
<td>OK unstable oscillations</td>
</tr>
<tr>
<td>upwind1 (58)</td>
<td>OK</td>
<td>OK stable oscillations</td>
<td>OK if CFL ≤ 0.3</td>
</tr>
<tr>
<td>upwind2 (59)</td>
<td>OK</td>
<td>OK stable oscillations</td>
<td>OK if CFL ≤ 0.2</td>
</tr>
</tbody>
</table>

Fig. 11. Task C3, \( M = 50 \), \( M = 200 \) and \( M = 800 \), details of comparison of Rusanov scheme (Rusanov) and scheme (77) with \( \epsilon = 1 \) (upw+e=1.0), \( \epsilon = 0.1 \) (upw+e=0.1) and \( \epsilon = 0.01 \) (upw+e=0.01), distributions of \( C_l \), \( C_s \) and \( P \) at \( t = 0.3 \).

Moreover, this figure shows that with the aid of a finer partitioning of \((0,L)\) we obtain a sharper capturing of discontinuities but the same retardation for each numerical scheme.

Therefore, we conclude that the Rusanov scheme and the schemes (77) with different parameter \( \epsilon \) converge to different “weak” solutions (differing by the speed of the propagation of the discontinuities). This is in agreement with the arguments presented in the introduction and Remark 2 of this paper, i.e., without additional (jump) conditions we can not define a proper weak solution and then different stable numerical schemes produce different solutions.
5 Conclusion

We dealt with the numerical simulation of steady and unsteady flows of solid-liquid-gas slurries which leads to a non-conservative system of equations with source terms. Although the non-conservativity and complexity of the problems prevent us to introduce the concept of a weak solution we solve this industrial challenging problem numerically. We tested several numerical schemes which are based on the solution of linearized Riemann problems and we observed that instabilities can cause a collapse of a calculation. These troubles may have the following reasons:

- unphysical intermediate states in the computation of the linearized Riemann problem,
- a high sensitivity of the non-conservative terms with respect to their arguments and consequently to the rounding errors namely when a no-vanishing eigenvalue of the matrix of the linearized Riemann problem has small magnitude.

On the other hand, the Rusanov scheme and the generalized upwind scheme with an artificial dissipative term (77) solved very well steady as well as unsteady flow problems. These schemes do not evaluate directly the solution of the linearized Riemann problem and therefore, do not suffer from the instability mentioned above. The presented numerical examples show that it is not easy to design a reliable numerical scheme for a non-conservative system ignoring completely the non-conservative equivalent of the Rankine-Hugoniot jump conditions. Various ‘upwind-like’ schemes produce different results and one has no clue as to what is physically reasonable, since the numerical results all look plausible. In order to obtain a unique solution, we have to add some extra information. There should be taken into account a more physical character of the model. This is a subject for the future research.

References


